

Home Done Solution

WIND STUDY

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Living Unit 27mq – BOX

Loads applied:

Dead load – structural, G1

Live load – floor, Q1= 2kN/m² = 42,8psf

Live load – roof, Q2=0,5kN/m² = 10,4psf

Wind load – according to Florida building code:

$$q = 0,00255 * k_z * k_{zt} * k_d * V^2 * I$$

Florida Building Code indicated general values of the coefficient:

I=1 for type 2 of construction (Residential Building)

K_{zt} =1 – topographic factor

K_z = 0,9 – velocity pressure exposure coefficient

K_d = 0,85 – wind directionally factor for buildings

Three different velocity of wind are analyzed:

$$V_1 = 150 \text{ mph}$$

$$V_2 = 200 \text{ mph}$$

$$V_3 = 250 \text{ mph}$$

$$V_4 = 300 \text{ mph}$$

Considering these values, the wind load is:

$$q_1 = 43,89 \text{ psf}$$

$$q_2 = 78,03 \text{ psf}$$

$$q_3 = 121,92 \text{ psf}$$

$$q_4 = 175,57 \text{ psf}$$

In conclusion, the design wind pressure is calculated by the following equation:

$$p = q_n * (Gc_{pf} - Gc_{pi})$$

Where:

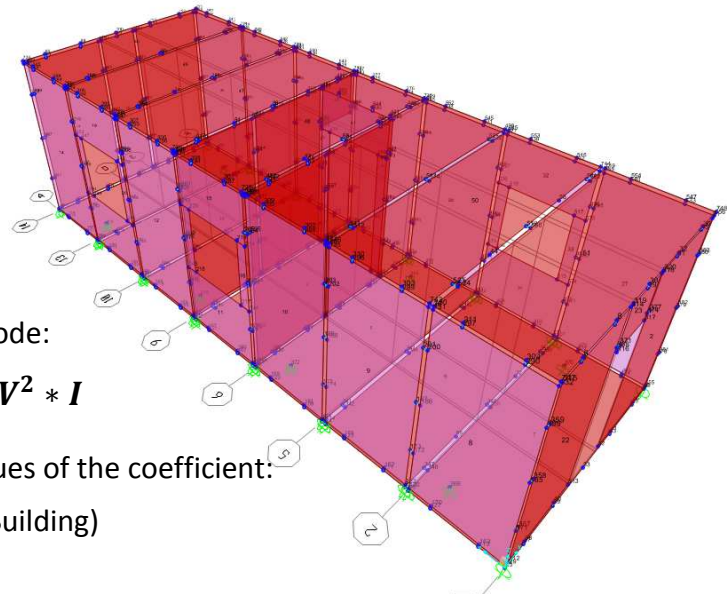
G_{c_{pf}}: external pressure coefficient from Figure 6-10 of ASCE7 code.

G_{c_{pi}} = ± 0,18 internal pressure coefficient

The combination factored loads used is:

$$1,2D + 1,6W + L + 0,5L_r$$

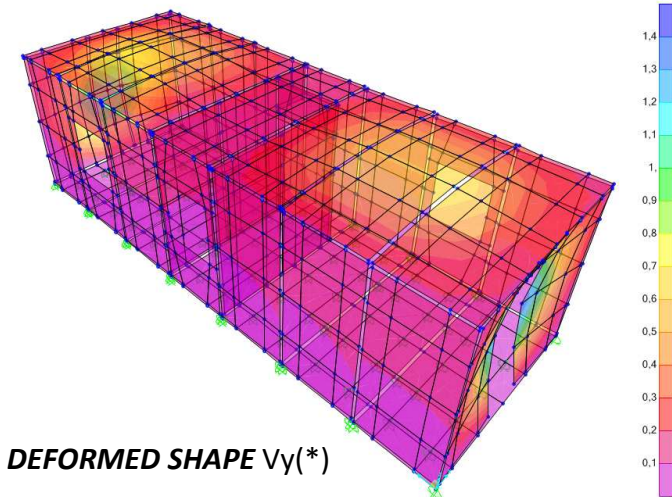
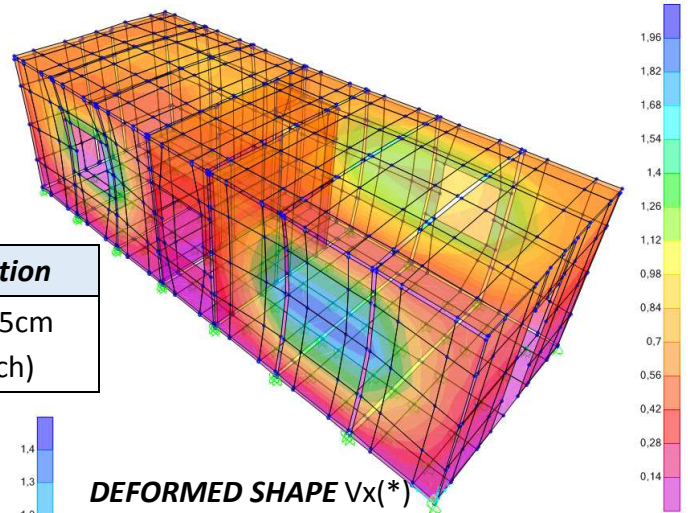
(*) The deformed shape obtained in output is amplified by 20-30 times to facilitate the identification of critical points, but the real deformation values can be read from the chromatic scale, measured in cm.



I. Case – V₁

Wall 2,60m x 1,20m – thickness 10 cm
 Floor/Roof 3,20m x 1,20m – thickness 18 cm

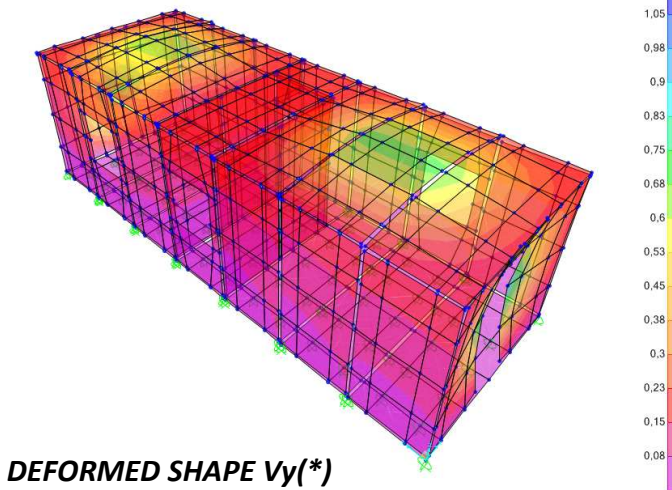
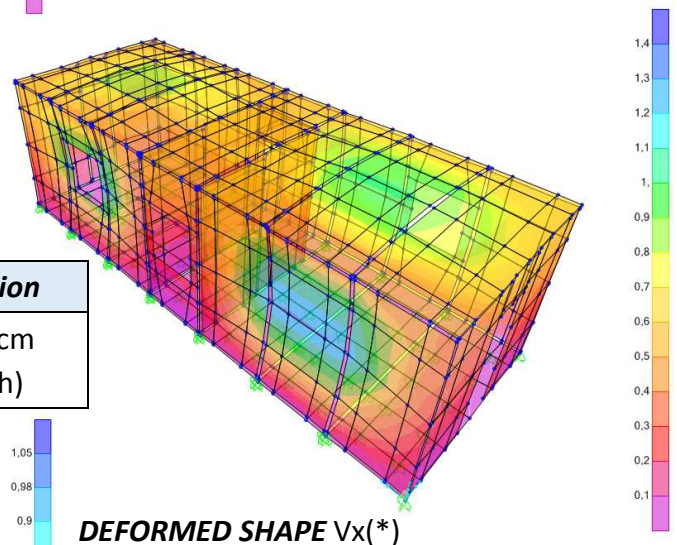
V _x	V _y	Combination
U _{max} = 1,96cm (0,77 inch)	U _{max} = 1,3cm (0,51 inch)	U _{max} = 3,95cm (1,56 inch)



II. Case – V₁

Wall 2,60m x 1,20m – thickness 14 cm
 Floor/Roof 3,20m x 1,20m – thickness 18 cm

V _x	V _y	Combination
U _{max} = 1,3cm (0,51 inch)	U _{max} = 0,98cm (0,39 inch)	U _{max} = 2,2cm (0,87 inch)

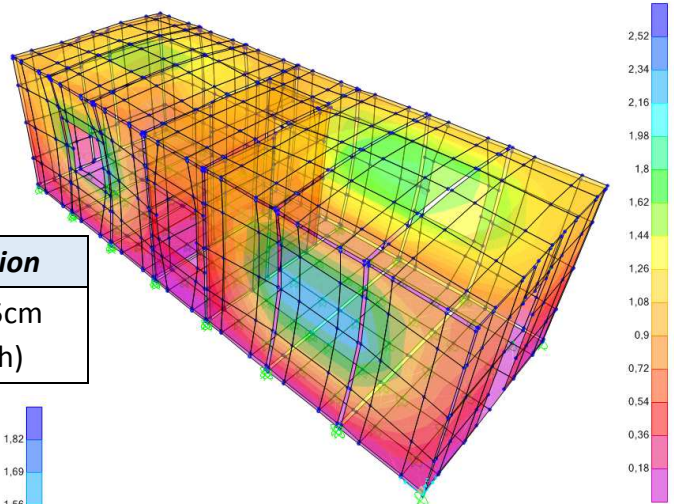


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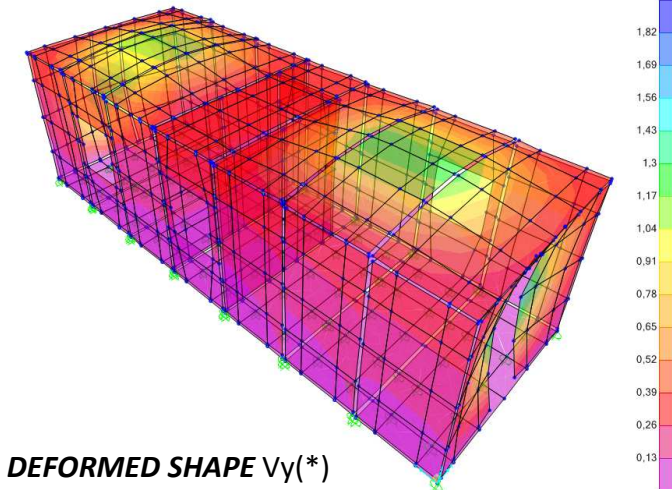
III. Case – V₂

Wall 2,60m x 1,20m – thickness 14 cm
 Floor/Roof 3,20m x 1,20m – thickness 18 cm

V _x	V _y	Combination
U _{max} = 2,34cm (0,92 inch)	U _{max} = 1,56cm (0,61 inch)	U _{max} = 3,85cm (1,52 inch)



DEFORMED SHAPE V_x(*)

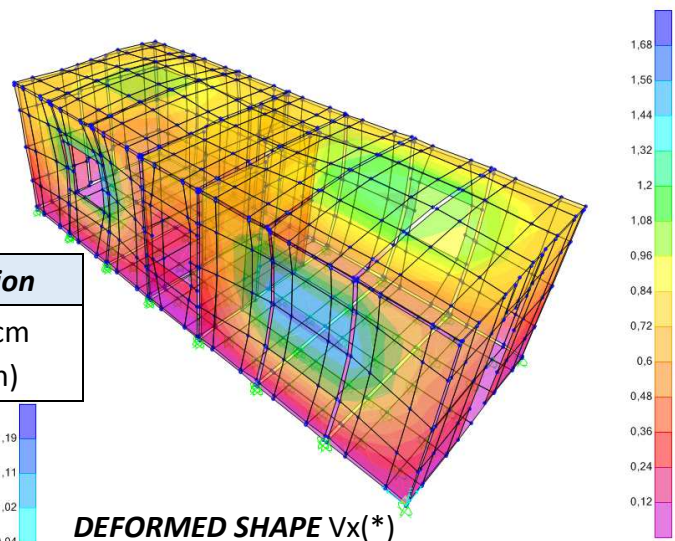


DEFORMED SHAPE V_y(*)

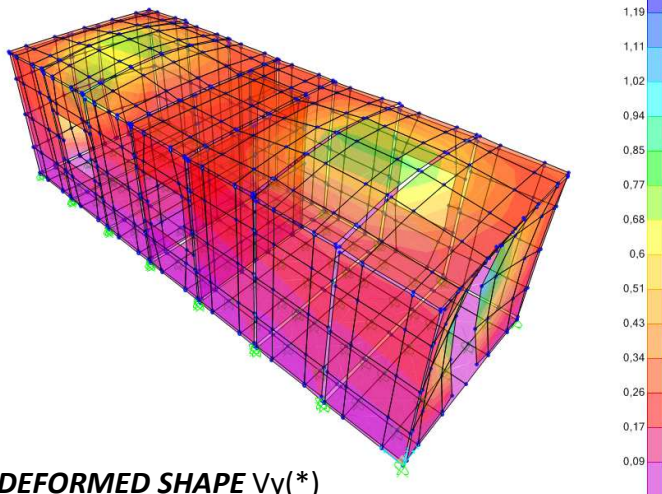
IV. Case – V₂

Wall 2,60m x 1,20m – thickness 16 cm
 Floor/Roof 3,20m x 1,20m – th= 18 cm – four grid

V _x	V _y	Combination
U _{max} = 1,56cm (0,61 inch)	U _{max} = 1,11cm (0,44 inch)	U _{max} = 2,5cm (0,98 inch)



DEFORMED SHAPE V_x(*)



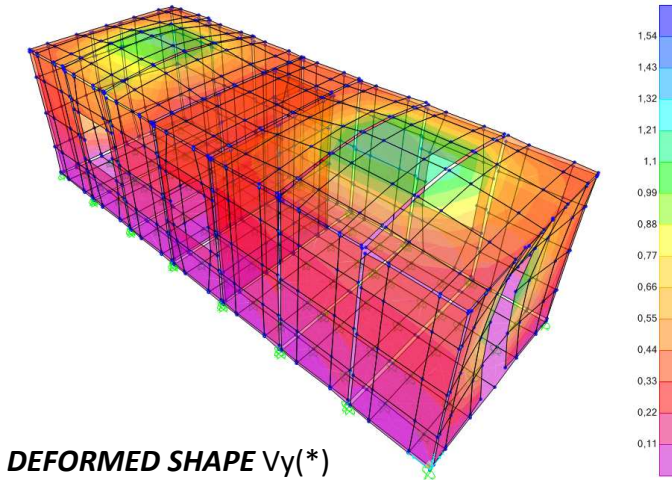
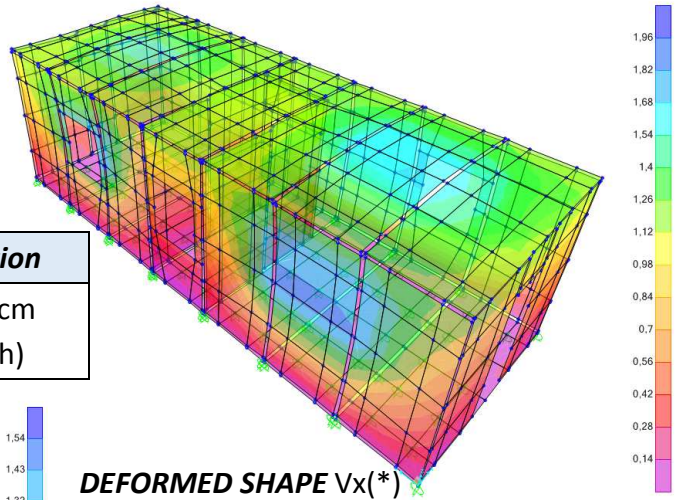
DEFORMED SHAPE V_y(*)

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V. Case – V₃

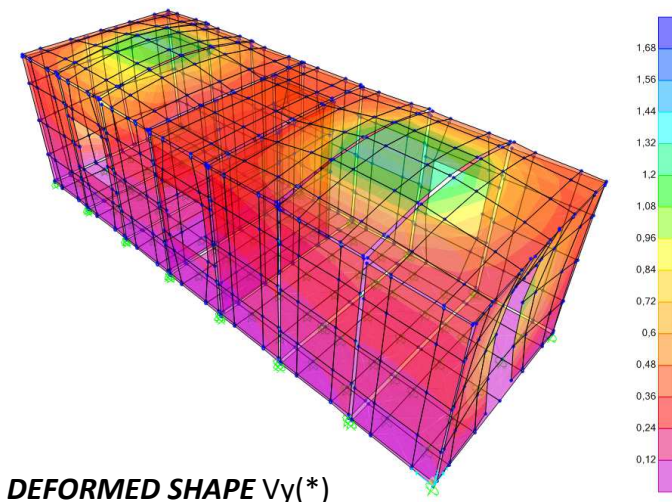
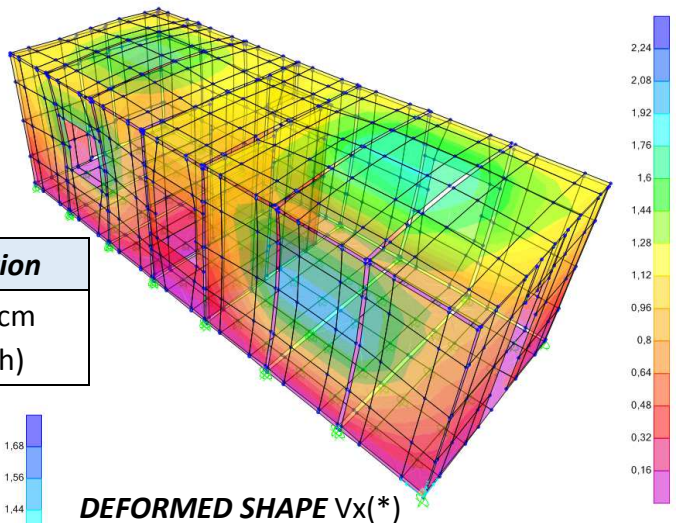
Wall 2,60m x 1,20m – thickness 18 cm
 Floor/Roof 3,20m x 1,20m – th= 20 cm – four grid

V _x	V _y	Combination
U _{max} = 1,82cm (0,72 inch)	U _{max} = 1,21cm (0,48 inch)	U _{max} = 3,5cm (1,38 inch)


VI. Case – V₄

Wall 2,60m x 1,20m – th= 18 cm – four grid
 Floor/Roof 3,20m x 1,20m – th= 20 cm – four grid

V _x	V _y	Combination
U _{max} = 1,92cm (0,76 inch)	U _{max} = 1,32cm (0,52 inch)	U _{max} = 4,1cm (1,61 inch)



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CONCLUSION

This study wants to demonstrate that with Home Done panels is possible to achieve the necessary results in terms of deformation and resistance, changing the thickness or the armors of the panels themselves.

The results obtained are:

- $V_1= 150\text{mph}$ -> Wall 14cm (5,51 inch) – Floor/Roof 18cm (7,09 inch)
- $V_2= 200\text{mph}$ -> Wall 16cm (6,3 inch) – Floor/Roof 18cm four grid (7,09 inch)
- $V_3= 250\text{mph}$ -> Wall 18cm (7,09 inch) – Floor/Roof 20cm four grid (7,87 inch)
- $V_4= 300\text{mph}$ -> Wall 18cm four grid (7,09 inch) – Floor/Roof 20cm four grid (7,87 inch)

The Home done system is a simple regular shape, best characteristics in determining the performance of buildings in cyclones, which avoid concentration of pressure in the structure. Most houses are rectangular and the best layout is when the length is not more than three times the width. In addition, the foundations, the walls, and the roof structure are all firmly fixed together.

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